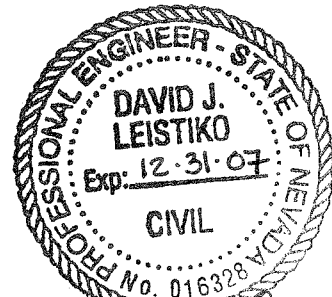


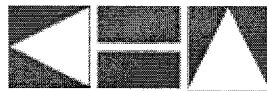
RETAINING WALL DESIGN CALCULATIONS

McLeod Walls

Prepared By:
Kimley-Horn and Associates, Inc.
7878 N. 16th Street
Suite 300
Phoenix, AZ 85020



David J. Leistiko
9/9/05



**Kimley-Horn
and Associates, Inc.**

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Criteria

Retained Height	=	6.50 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00 : 1
Height of Soil over Toe	=	12.00 in
Water height over heel	=	0.0 ft
Wind on Stem	=	0.0 psf
Soil height to ignore for passive pressure	=	12.00 in

Soil Data

Allow Soil Bearing	=	3,000.0 psf
Equivalent Fluid Pressure Method	=	1.25 ft
Heel Active Pressure	=	65.0 psf/ft
Toe Active Pressure	=	300.0 psf/ft
Passive Pressure	=	110.00 pcf
Soil Density	=	0.500
Footings/Soil Friction	=	0.500
Soil height to ignore for passive pressure	=	12.00 in

Footing Strengths & Dimensions

Toe Width	=	1.25 ft
Heel Width	=	4.00 ft
Total Footing Width	=	5.25 ft
Footing Thickness	=	18.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	3.75 ft
f'c = 3,000 psi	Fy =	60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top = 2.00 in @ Btm = 3.00 in		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
Surcharge Over Toe	=	0.00 ft
Used for Sliding & Overturning	=	0.00 ft

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Fig CL Dist	=	0.00 ft
Footing Type	=	0.0 ft
Base Above/Below Soil at Back of Wall	=	0.300
Poisson's Ratio	=	0.300
Added seismic base force	=	2,130.5 lbs

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Design Kh	=	0.050 g
Using Mononobe-Okabe / Seed-Whitman procedure		
Kae for seismic earth pressure	=	1.052
Ka for static earth pressure	=	0.710
Difference: Kae - Ka	=	0.343

Stem Weight Seismic Load

F _y / V _p Weight Multiplier	=	0.100 g
Added seismic base force	=	0.0 lbs

Tapered Concrete Stem Design Data

Thickness at TOP	=	8.00 in
Thickness at BOTTOM	=	12.00 in
Rebar Cover (rebar center to concrete face 2.00 in)		
Design Height Above Ft _c	=	3.00 ft
Rebar Size	=	# 5
Rebar Spacing	=	18.00 in
Rebar Depth 'd'	=	7.65 in

Design Summary

Total Bearing Load	=	4,667 lbs
...resultant ecc.	=	6.87 in
Soil Pressure @ Toe	=	1,640 psf OK
Soil Pressure @ Heel	=	138 psf OK
Allowable	=	3,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,803 psf
ACI Factored @ Heel	=	152 psf
Footing Shear @ Toe	=	8.3 psi OK
Footing Shear @ Heel	=	10.5 psi OK
Allowable	=	82.2 psi
Sliding Stability Ratio	=	1.50 OK
Sliding Cales (Vertical Component NOT Used)		
Lateral Sliding Force	=	1,986.3 lbs
less 100% Passive Forces	=	787.5 lbs
less 100% Friction Force =	=	2,197.7 lbs
Added Force Req'd	=	0.0 lbs OK
...for 1.5 : 1 Stability	=	0.0 lbs OK

Load Factors

Building Code	Other
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Footing Design Results

Factored Pressure	=	1,803
Mir : Upward	=	1,660
Mir : Downward	=	403
Mir : Design	=	1,257
Actual 1-Way Shear	=	8.33
Allow 1-Way Shear	=	82.16 psi
Toe Reinforcing	=	# 5 @ 18.00 in
Heel Reinforcing	=	# 5 @ 18.00 in
Key Reinforcing	=	# 6 @ 18.00 in
Other Acceptable Sizes & Spacings		
Toe: Not req'd, Mu < S * Fr		
Heel: Not req'd, Mu < S * Fr		
Key: No key defined		

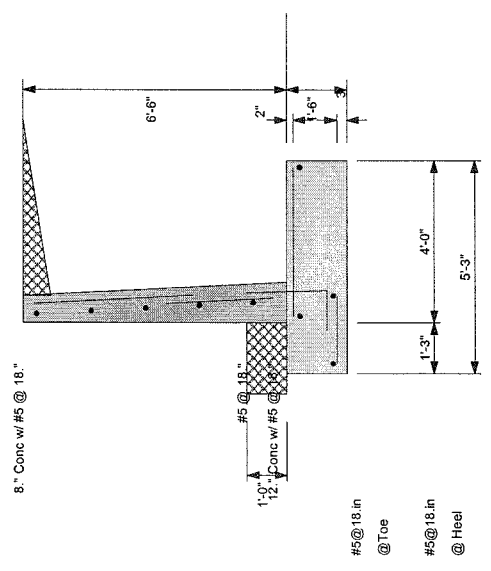
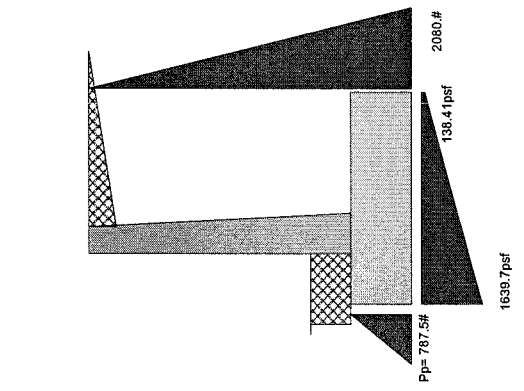
Summary of Overturning & Resisting Forces & Moments

Item	Force lbs	Distance ft	Moment ft-lb	
Heel Active Pressure	=	2,080.0	2.67	5,546.7
Toe Active Pressure	=	-93.8	0.83	-78.1
Surcharge Over Toe	=			
Adjacent Footing Load	=			
Added Lateral Load	=			
Load @ Stem Above Soil	=	2,130.5	0.05	107.3
Seismic Load	=			
Total	=	1,986.3	O.T.M. =	5,468.5
Resisting/Overturning Ratio	=			2.61
Vertical Loads used for Soil Pressure	=			4,667.5 lbs
Vertical component of active pressure used for soil pressure	=			272.1 lbs
Total	=			4,667.5 lbs
R.M.I. =				14,272.5

DESIGNER NOTES:

Vertical component of active pressure used for soil pressure





Kimley-Horn and Associates, Inc.
7878 N. 16th Street
Suite 300
Phoenix, AZ 85020

Title : McLeod 2:1 backfill
Job # : TBD 8.5-ft
Description :
Dsgnr: IBC 2000 Date: SEP 2, 2005

Retain Pro 2005, 7-April-2005, (c) 1989-2005
www.retainpro.com/report for latest release
Registration # : RP-114815 205001

This Wall in File: K:\Rdwy\Structural\Las Vegas\McLeod\m

Tapered Stem Concrete Retaining Wall Design

Criteria	
Retained Height	= 8.33 ft
Wall height above soil	= 0.00 ft
Slope Behind Wall	= 0.00 : 1
Height of Soil over Toe	= 12.00 in
Water height over heel	= 0.0 ft
Wind on Stem	= 0.0 psf

Soil Data	
Allow Soil Bearing	= 3,000.0 psf
Equivalent Fluid Pressure Method	= 2.50 ft
Heel Active Pressure	= 65.0 psf/ft
Toe Active Pressure	= 30.0 psf/ft
Passive Pressure	= 110.00 pcf
Soil Density	= 0.500
Footing/Soil Friction	= 12.00 in
Soil height to ignore for passive pressure	= 0.0018

Lateral Load Applied to Stem	
Lateral Load	= 0.0 #ft
...Height to Top	= 0.00 ft
...Height to Bottom	= 0.00 ft

Surcharge Loads	
Surcharge Over Heel	= 0.0 psf
>>>Used to Resist Sliding & Overturning	
Surcharge Over Toe	= 0.0 psf
Used for Sliding & Overturning	

Axial Load Applied to Stem	
Axial Dead Load	= 0.0 lbs
Axial Live Load	= 0.0 lbs
Axial Load Eccentricity	= 0.0 in

Earth Pressure Seismic Load	
Design K ₁	= 0.050 g

Using Mononobe-Okabe / Seed-Whitman procedure
F_v / V_o Weight Multiplier = 0.100 g Added seismic base force

Design Summary	
Total Bearing Load	= 6,243 lbs
...resultant ecc.	= 9.14 in
Soil Pressure @ Toe	= 1,636 psf OK
Soil Pressure @ Heel	= 285 psf OK
Allowable	= 3,000 psf
Soil Pressure Less Than Allowable	
ACI Factored @ Toe	= 1,786 psf
ACI Factored @ Heel	= 311 psf
Footing Shear @ Toe	= 15.2 psi OK
Footing Shear @ Heel	= 15.4 psi OK
Allowable	= 82.2 psi OK
Sliding Stability Ratio	= 1.51 OK
Sliding Cofcs (Vertical Component NOT Used)	
Lateral Sliding Force	= 3,048.6 lbs
less 100% Passive Force	= - 1,687.5 lbs
Added Friction Force	= - 2,915.9 lbs
Added Force Res'd	= 0.0 lbs OK
...for 1.5 : 1 Stability	= 0.0 lbs OK

Load Factors	
Building Code	Other
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Soil Data	
Allow Soil Bearing	= 3,000.0 psf
Equivalent Fluid Pressure Method	= 2.50 ft
Heel Active Pressure	= 65.0 psf/ft
Toe Active Pressure	= 30.0 psf/ft
Passive Pressure	= 110.00 pcf
Soil Density	= 0.500
Footing/Soil Friction	= 12.00 in
Soil height to ignore for passive pressure	= 0.0018

Lateral Load Applied to Stem	
Lateral Load	= 0.0 #ft
...Height to Top	= 0.00 ft
...Height to Bottom	= 0.00 ft

Surcharge Loads	
Surcharge Over Heel	= 0.0 psf
>>>Used to Resist Sliding & Overturning	
Surcharge Over Toe	= 0.0 psf
Used for Sliding & Overturning	

Axial Load Applied to Stem	
Axial Dead Load	= 0.0 lbs
Axial Live Load	= 0.0 lbs
Axial Load Eccentricity	= 0.0 in

Earth Pressure Seismic Load	
Design K ₁	= 0.050 g

Using Mononobe-Okabe / Seed-Whitman procedure
F_v / V_o Weight Multiplier = 0.100 g Added seismic base force

Design Summary	
Thickness at TOP	= 8.00 in
Thickness at BOTTOM	= 12.00 in
Fy	= 60,000 psi
Rebar Cover (rebar center to concrete face 2.00 in	
Rebar Spacing	= 18.00 in
Rebar Depth 'd'	= 9.02 in
Design Data	
M _u ...Actual	= 2,629.0 ft-lb
M _n * Phi...Allowable	= 7,306.7 ft-lb
Shear Force @ this height	= 1,478.9 lbs
V _u ...Actual	= 15.29 psi
V _n * Phi...Allowable	= 82.16 psi
Rebar Lap Required	= 21.36 in
Hooked embedment into footing (w/ stress level reduction)	= 6.00 in

Footing Strengths & Dimensions	
Toe Width	= 2.50 ft
Heel Width	= 6.50
Total Footing Width	= 9.00
Footing Thickness	= 18.00 in
Key Width	= 12.00 in
Key Depth	= 12.00 in
Key Distance from Toe	= 4.00 ft
f _c = 3,000 psi	F _y = 60,000 psi
Footing Concrete Density	= 150.00 pcf
Min. As %	= 0.0018
Cover @ Top = 2.00 in @ Btm = 3.00 in	

Adjacent Footing Load	
Adjacent Footing Load	= 0.0 lbs
Footing Width	= 0.00 ft
Eccentricity	= 0.00 in
Wall to Fig CL Dist	= 0.00 ft
Footing Type	
Base Above/Below Soil	= 0.0 ft
at Back of Wall	
Poisson's Ratio	= 0.300
Added seismic base force	= 2,130.5 lbs

Vertical Component	
Added seismic base force	= 2,130.5 lbs
Vertical component of active pressure	= 6,242.8 lbs

Design Summary	
Thickness at TOP	= 8.00 in
Thickness at BOTTOM	= 12.00 in
Fy	= 60,000 psi
Rebar Cover (rebar center to concrete face 2.00 in	
Rebar Spacing	= 18.00 in
Rebar Depth 'd'	= 9.02 in
Design Data	
M _u ...Actual	= 2,629.0 ft-lb
M _n * Phi...Allowable	= 7,306.7 ft-lb
Shear Force @ this height	= 1,478.9 lbs
V _u ...Actual	= 15.29 psi
V _n * Phi...Allowable	= 82.16 psi
Rebar Lap Required	= 21.36 in
Hooked embedment into footing (w/ stress level reduction)	= 6.00 in

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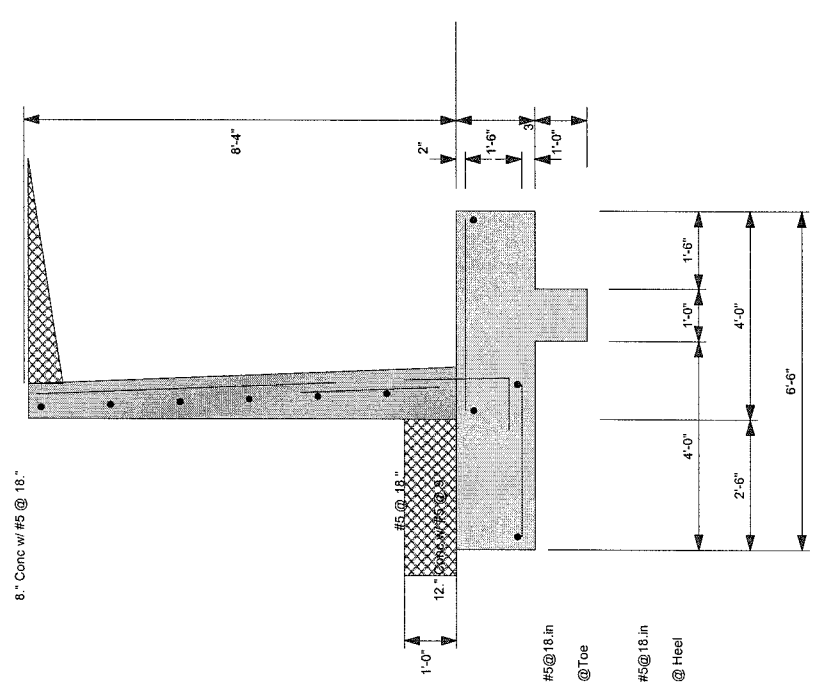
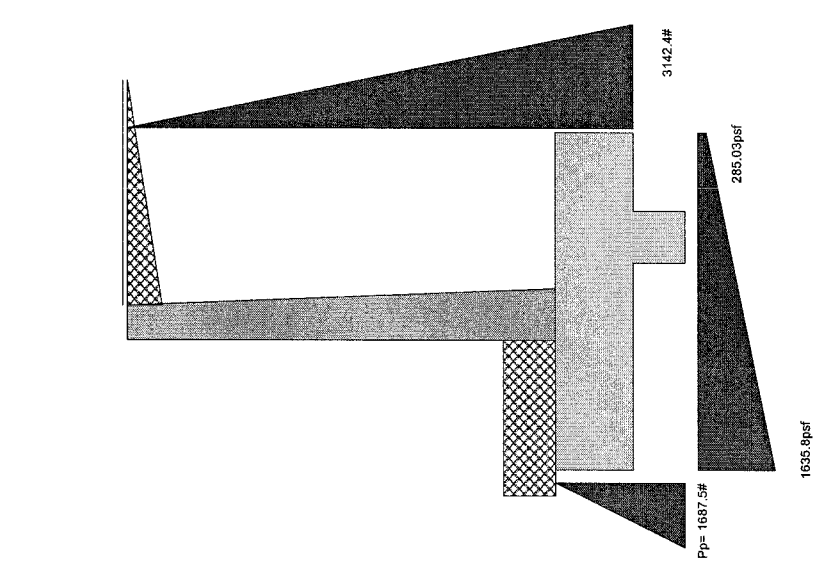
Tapered Stem Concrete Retaining Wall Design

Footing Design Results	
Factored Pressure	= 1,786
M _u : Upward	= 5,932
M _u : Downward	= 1,429
M _u : Design	= 4,203
Actual 1-Way Shear	= 15.17
Allow 1-Way Shear	= 82.16 psi
Key:	
Toe Reinforcing	= # 5 @ 18.00 in
Heel Reinforcing	= # 5 @ 18.00 in
Key Reinforcing	= # 5 @ 18.00 in
Other Acceptable Sizes & Spacings	
Toe: Not req'd, M _u < S * F _r	
Heel: Not req'd, M _u < S * F _r	

Summary of Overturning & Resisting Forces & Moments			
Item	Force	Distance	Moment
Heel Active Pressure	= 3,142.4	3.28	10,299.6
Toe Active Pressure	= -93.8	0.83	-78.1
Surcharge Over Toe	=	=	=
Adjacent Footing Load	=	=	=
Added Lateral Load	=	=	=
Load @ Stem Above Soil	= 2,130.5	0.00	0.1
Seismic Load	=	=	=
Total	= 3,048.6	O.T.M. =	10,221.5

Resisting/Overturning Ratio	
Vertical Loads used for Soil Pressure	= 6,242.8 lbs
Vertical component of active pressure used for soil pressure	= 6,242.8 lbs
Resisting/Overturning Ratio	= 2.52
Total	= 6,242.8 lbs
Vertical component of active pressure used for soil pressure	= 6,242.8 lbs
Resisting/Overturning Ratio	= 2.52

Designer Notes:	
Vertical component of active pressure used for soil pressure	= 6,242.8 lbs
Vertical component of active pressure used for soil pressure	= 6,242.8 lbs
Resisting/Overturning Ratio	= 2.52
Total	= 6,242.8 lbs
Vertical component of active pressure used for soil pressure	= 6,242.8 lbs



Soil Data

Allow Soil Bearing	=	3,000.0	psf
Equivalent Fluid Pressure Method	=	0.0	ft
Heel Active Pressure	=	65.0	psf/ft
Toe Active Pressure	=	300.0	psf/ft
Passive Pressure	=	110.00	pcf
Soil Density	=	0.500	
Footings Soil Friction	=	12.00	in
Soil height to ignore for passive pressure	=	0.00	ft

Footing Strengths & Dimensions

Toe Width	=	4.00	ft
Heel Width	=	4.00	ft
Total Footing Width	=	8.00	ft
Footing Thickness	=	24.00	in
Key Width	=	18.00	in
Key Depth	=	18.00	in
Key Distance from Toe	=	3.75	ft
f'c	=	3,000	psi
Fy	=	60,000	psi
Footing Concrete Density	=	150.00	pcf
Min. As %	=	0.0018	
Cover @ Top	=	2.00	in @ Btm= 3.00

Lateral Load Applied to Stem

Adjacent Footing Load	=	0.0	lbs
Footing Width	=	0.00	ft
Eccentricity	=	0.00	in
Wall to Fig CL Dist	=	0.00	ft
Footing Type	=	0.0	ft
Base Above/Below Soil at Back of Wall	=	0.00	
Poisson's Ratio	=	0.300	
Added seismic base force	=	2,130.5	lbs

Surcharge Loads

Surcharge Over Heel	=	0.0	psf
Surcharge Over Toe	=	0.0	psf
Used for Sliding & Overturning	=	0.0	psf

Earth Pressure Seismic Load

Design Kh	=	0.050	g
Using Mononobe-Okabe / Seed-Whitman procedure	=	0.100	g

Design Summary

Total Bearing Load	=	8,523	lbs
...resultant ecc.	=	11.13	in
Soil Pressure @ Toe	=	1,807	psf OK
Soil Pressure @ Heel	=	324	psf OK
Allowable	=	3,000	psf
Soil Pressure Less Than Allowable	=	1,966	psf
ACI Factored @ Toe	=	16.7	psi OK
ACI Factored @ Heel	=	15.7	psi OK
Footing Shear @ Toe	=	82.2	psi OK
Footing Shear @ Heel	=	15.7	psi OK
Allowable	=	1.51	OK
Sliding Stability Ratio	=	4.545	OK
Lateral Sliding Force	=	2,887.5	lbs
less 100% Passive Force	=	-3,955.4	lbs
Added Force Req'd	=	0.0	lbs OK
...for 1.5 : 1 Stability	=	0.0	lbs OK

Design Summary (cont)

Building Code	=	Other
Dead Load	=	1,200
Live Load	=	1,600
Earth, H	=	1,600
Wind, W	=	1,600
Seismic, E	=	1,000

Footings Design Results

Factored Pressure	=	1,966	psf
Mu: Upward	=	14.832	ft-k
Mu: Downward	=	4.271	ft-k
Mu: Design	=	10.361	ft-k
Actual 1-Way Shear	=	16.69	psi
Allow 1-Way Shear	=	82.16	psi
Toe Reinforcing	=	# 6 @ 12.00	in
Heel Reinforcing	=	# 6 @ 18.00	in
Key Reinforcing	=	# 6 @ 18.00	in
Other Acceptable Sizes & Spacings	=		
Toe: Not req'd, Mu < S * Fr	=		
Heel: Not req'd, Mu < S * Fr	=		
Key: Not req'd = Mu < S * Fr	=		

Summary of Overturning & Resisting Forces & Moments

Item	Force lbs	Distance ft	Moment ft-k
Soil Over Heel	=	3,300.0	=
Sloped Soil Over Heel	=		
Surcharge Over Heel	=		
Adjacent Footing Load	=		
Added Lateral Load	=		
Load @ Stem Above Soil	=		
Seismic Load	=		
Total	=		

Summary of Overturning & Resisting Forces & Moments (cont)

Item	Force lbs	Distance ft	Moment ft-k
Heel Active Pressure	=	4,680.0	=
Toe Active Pressure	=	-135.0	=
Surcharge Over Toe	=		
Adjacent Footing Load	=		
Added Lateral Load	=		
Load @ Stem Above Soil	=		
Seismic Load	=		
Total	=		

Design Summary (cont)

Vertical Loads used for Soil Pressure	=	8,523.0	lbs
Vertical component of active pressure used for soil pressure	=	8,523.0	lbs

Summary of Overturning & Resisting Forces & Moments (cont)

Item	Force lbs	Distance ft	Moment ft-k
Soil Over Heel	=	3,300.0	=
Sloped Soil Over Heel	=		
Surcharge Over Heel	=		
Adjacent Footing Load	=		
Added Lateral Load	=		
Load @ Stem Above Soil	=		
Seismic Load	=		
Total	=		

Design Summary (cont)

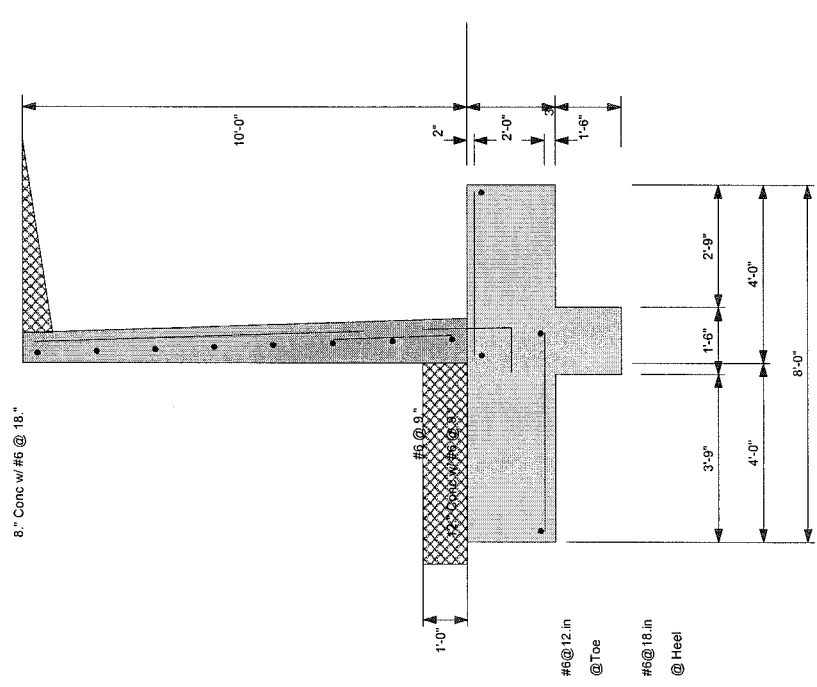
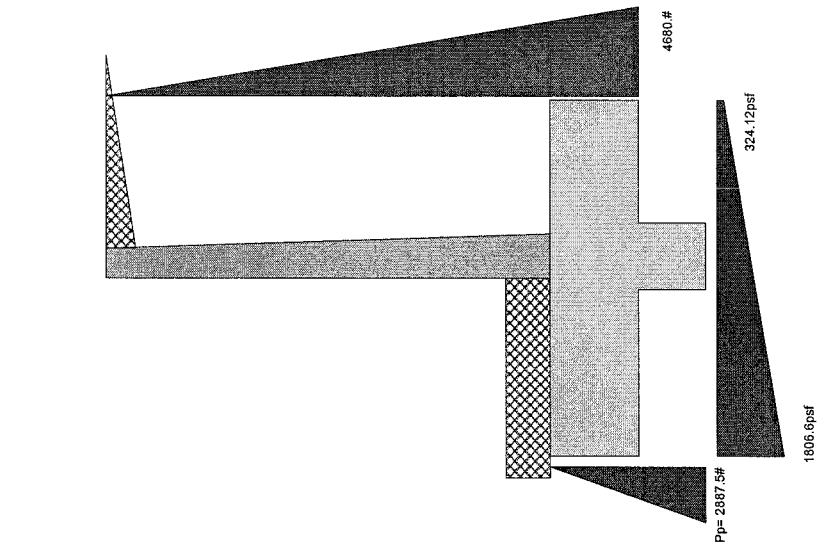
Vertical Loads used for Soil Pressure	=	8,523.0	lbs
Vertical component of active pressure used for soil pressure	=	8,523.0	lbs

Design Summary (cont)

Vertical component of active pressure used for soil pressure	=	8,523.0	lbs
Vertical component of active pressure used for soil pressure	=	8,523.0	lbs

Design Summary (cont)

Vertical component of active pressure used for soil pressure	=	8,523.0	lbs
Vertical component of active pressure used for soil pressure	=	8,523.0	lbs



Tapered Stem Concrete Retaining Wall Design

Soil Data

Retained Height	= 11.33 ft
Wall height above soil	= 0.00 ft
Slope Behind Wall	= 0.00 : 1
Height of Soil over Toe	= 12.00 in
Water height over heel	= 0.0 ft
Wind on Stem	= 0.0 psf
Soil height to ignore for passive pressure	= 0.00 in

Soil Strengths & Dimensions

Toe Width	= 4.00 ft
Heel Width	= 8.00
Total Footing Width	= 12.00 in
Footing Thickness	= 24.00 in
Key Width	= 18.00 in
Key Depth	= 28.00 in
Key Distance from Toe	= 5.00 ft
Fc = 3,000 psi	Fy = 60,000 psi
Footing Concrete Density = 150.00 pcf	Mix. As % = 0.0018
Cover @ Top = 2.00 in	@ Btm = 3.00 in

Lateral Load Applied to Stem

Lateral Load	= 0.0 #/ft
Height to Top	= 0.00 ft
Height to Bottom	= 0.00 ft

Adjacent Footing Load

Adjacent Footing Load	= 0.0 lbs
Footing Width	= 0.00 ft
Eccentricity	= 0.00 in
Wall to Fig CL Dist	= 0.00 ft
Footing Type	= 0.0 ft
Base Above/Below Soil at Back of Wall	= 0.300
Poisson's Ratio	= 0.300
Added seismic base force	= 2,130.5 lbs

Earth Pressure Seismic Load

Design Kh	= 0.050 g
Using Mononobe-Okabe / Seed-Whitman procedure	
Kae for seismic earth pressure	= 0.1092
Ka for static earth pressure	= 0.710
Difference: Kae - Ka	= 0.343

Design Summary

Total Bearing Load	= 9,485.0 lbs
...resultant ecc.	= 15.62 in
Soil Pressure @ Toe	= 2,343 psf OK
Soil Pressure @ Heel	= 28 psf OK
Soil Pressure Less Than Allowable	= 3,000 psf
ACI Factored @ Toe	= 2,526 psf
ACI Factored @ Heel	= 30 psf
Footing Shear @ Toe	= 22.2 psf OK
Footing Shear @ Heel	= 20.5 psf OK
Allowable	= 82.2 psf

Sliding Stability Ratio

Mu...Actual	= 10,029.7 ft#
Mn * Phi...Allowable	= 14,486.1 ft#
Shear Force @ this height	= 3,610.8 lbs
Vu...Actual	= 35.65 psf
Vn * Phi...Allowable	= 82.16 psf
Added Force Req'd	= 37.38 in
...for 1.5 : 1 Stability	= 37.38 in

Load Factors

Building Code	Other
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Design Summary

Thickness at TOP	= 8.00 in
Thickness at BOTTOM	= 12.00 in
Rebar Cover (rebar center to concrete face 2.00 in	
@ Height #1	Stem OK
@ Height #2	Stem OK
@ Base of Wall	Stem OK
Design Height Above Ftg	= 3.00 ft
Rebar Size	= 7
Rebar Spacing	= 18.00 in
Rebar Depth 'd'	= 8.44 in
Design Data	
Mu...Actual	= 10,029.7 ft#
Mn * Phi...Allowable	= 14,486.1 ft#
Shear Force @ this height	= 3,610.8 lbs
Vu...Actual	= 35.65 psf
Vn * Phi...Allowable	= 82.16 psf
Added Force Req'd	= 37.38 in
...for 1.5 : 1 Stability	= 37.38 in

Design Summary

Total Bearing Load	= 9,485.0 lbs
...resultant ecc.	= 15.62 in
Soil Pressure @ Toe	= 2,343 psf OK
Soil Pressure @ Heel	= 28 psf OK
Soil Pressure Less Than Allowable	= 3,000 psf
ACI Factored @ Toe	= 2,526 psf
ACI Factored @ Heel	= 30 psf
Footing Shear @ Toe	= 22.2 psf OK
Footing Shear @ Heel	= 20.5 psf OK
Allowable	= 82.2 psf

Sliding Stability Ratio

Mu...Actual	= 10,029.7 ft#
Mn * Phi...Allowable	= 14,486.1 ft#
Shear Force @ this height	= 3,610.8 lbs
Vu...Actual	= 35.65 psf
Vn * Phi...Allowable	= 82.16 psf
Added Force Req'd	= 37.38 in
...for 1.5 : 1 Stability	= 37.38 in

Load Factors

Building Code	Other
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Design Summary

Thickness at TOP	= 8.00 in
Thickness at BOTTOM	= 12.00 in
Rebar Cover (rebar center to concrete face 2.00 in	
@ Height #1	Stem OK
@ Height #2	Stem OK
@ Base of Wall	Stem OK
Design Height Above Ftg	= 3.00 ft
Rebar Size	= 7
Rebar Spacing	= 18.00 in
Rebar Depth 'd'	= 8.44 in
Design Data	
Mu...Actual	= 10,029.7 ft#
Mn * Phi...Allowable	= 14,486.1 ft#
Shear Force @ this height	= 3,610.8 lbs
Vu...Actual	= 35.65 psf
Vn * Phi...Allowable	= 82.16 psf
Added Force Req'd	= 37.38 in
...for 1.5 : 1 Stability	= 37.38 in

Load Factors

Building Code	Other
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

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Tapered Stem Concrete Retaining Wall Design

Footings Design Results

Factored Pressure	Toe	Heel
Min. Upward	2,950	0 psf
Min. Downward	20,727	966 psf
Actual 1-Way Shear	4,271	13,849 ft-#
Allow 1-Way Shear	16,456	12,853 ft-#
	26.09	23.86 psf
	82.16	82.16 psf

Toe Reinforcing = # 6 @ 12.00 in
 Heel Reinforcing = # 6 @ 18.00 in
 Key Reinforcing = # 6 @ 18.00 in
 Other Acceptable Sizes & Spacings
 Toe: #4 @ 5.50 in, #5 @ 8.50 in, #6 @ 12.00 in, #7 @ 16.50 in, #8 @ 21.50 in, #9 @ 27.00 in
 Heel: Not req'd, Mu < 2 * Fr
 Key:

Summary of Overturning & Resisting Forces & Moments

Item	Force lbs	Overturning Distance ft	Moment ft-#	Force lbs	Resisting Distance ft	Moment ft-#
Heel Active Pressure	6,445.8	4.69	30,258.5			
Toe Active Pressure	-135.0	1.00	-135.0			
Surcharge Over Toe						
Adjacent Footing Load						
Added Lateral Load						
Load @ Stem Above Soil	2,130.5	0.00	0.0	440.0	2.00	880.0
Seismic Load						
Total	6,310.8	O.T.M. =	30,123.5	1,510.4	4.42	6,679.2
				221.5	4.89	1,083.0
				2,400.0	4.00	9,600.0
				618.8	5.75	3,557.8
				843.2	8.00	6,745.3
				Total =	10,021.2	54,463.4

Resisting/Overturning Ratio = 1.81
 Vertical Loads used for Soil Pressure = 10,021.2 lbs
 Vertical component of active pressure used for soil pressure

DESIGNER NOTES:

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Tapered Stem Concrete Retaining Wall Design

Soil Data

Allow Soil Bearing	3,000.0 psf
Equivalent Fluid Pressure Method	4.00 ft
Heel Active Pressure	65.0 psf/ft
Toe Active Pressure	30.0 psf/ft
Passive Pressure	300.0 psf/ft
Soil Density	110.00 pcf
Footings Soil Friction	0.500
Soil height to ignore for passive pressure	0.00 in

Footings Strengths & Dimensions

Toe Width	4.00 ft
Heel Width	8.00 ft
Total Footing Width	24.00 in
Footing Thickness	18.00 in
Key Depth	33.00 in
Key Distance from Toe	5.00 ft
f'c =	3,000 psi
Fy =	60,000 psi
Footing Concrete Density	150.00 pcf
Min. As %	0.0018
Cover @ Top	2.00 in @ Btm. = 3.00 in

Lateral Load Applied to Stem

Lateral Load	0.0 #/ft
Height to Top	0.00 ft
Height to Bottom	0.00 ft

Adjacent Footing Load

Adjacent Footing Load	0.0 lbs
Footing Width	0.00 ft
Eccentricity	0.00 in
Wall to Fig CL Dist	0.00 ft
Footing Type	
Base Above/Below Soil	0.0 ft
Poisson's Ratio	0.300
Added seismic base force	2,130.5 lbs

Ka for seismic earth pressure = 1.052
 Ka for static earth pressure = 0.710
 Difference: Ka - Ka = 0.343
 Fp / Wp Weight Multiplier = 0.100 g Added seismic base force = 0.0 lbs

Design Summary

Total Bearing Load	10,021 lbs
...resultant ecc.	15.89 in
Soil Pressure @ Toe	2,751 psf OK
Soil Pressure @ Heel	0 psf OK
Soil Pressure Less Than Allowable	3,000 psf
ACI Factored @ Toe	2,950 psf
ACI Factored @ Heel	0 psf
Footing Shear @ Toe	26.1 psi OK
Footing Shear @ Heel	23.9 psi OK
Allowable	82.2 psi
Sliding Stability Ratio	1.51 OK
Sliding Cales (Vertical Component NOT Used)	
Lateral Sliding Force	6,310.8 lbs
less 100% Passive Force	4,959.4 lbs
less 100% Friction Force	4,589.0 lbs
Added Force Req'd	0.0 lbs OK
...for 1.5 : 1 Stability	0.0 lbs OK

Tapered Concrete Stem Design Data

Thickness at TOP	8.00 in	Fy =	60,000 psi	
Thickness at BOTTOM	12.00 in	Fc =	3,000 psi	
Rebar Cover (rebar center to concrete face)	2.00 in			
		@ Height #1	@ Height #2	@ Base of Wall
Design Data		Stem OK	Stem OK	Stem OK
Rebar Size	3.00 ft	1.00 ft	0.00 ft	
Rebar Spacing	# 7	# 7	# 7	
Rebar Depth 'd'	18.00 in	9.00 in	9.00 in	
Design Data		9.17 in	9.50 in	
Mu...Actual	12.9888 ft-#			
Mn * Phi...Allowable	14,604.4 ft-#			
Shear Force @ this height	4,280.0 lbs			
Vu...Actual	6,387.3 lbs			
Vu...Phi...Allowable	42.03 psi			
Rebar Lap Required	82.16 psi			
Hooked embedment into footing (w/ stress level reduction)	37.38 in			
	37.38 in			
				13.04 in

Load Factors

Building Code	Other
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

